



EULER-BERNOULLI AND TIMOSHENKO BEAM THEORIES

CONTENTS

Euler-Bernoulli beam theory

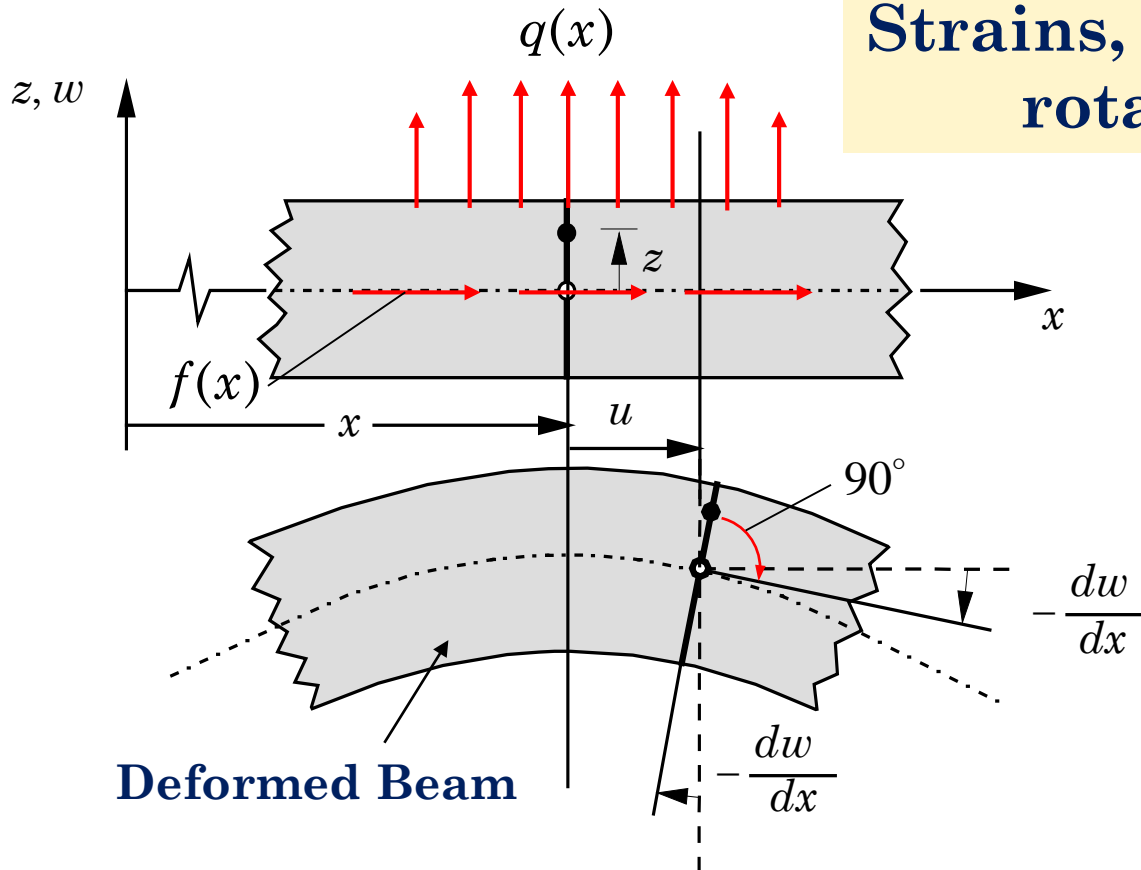
- Kinematics
- Equilibrium equations
- Governing equations in terms of the displacements

Timoshenko beam theory

- Kinematics
- Equilibrium equations
- Governing equations in terms of the displacements

KINEMATICS OF THE LINEARIZED EULER-BERNOULLI BEAM THEORY

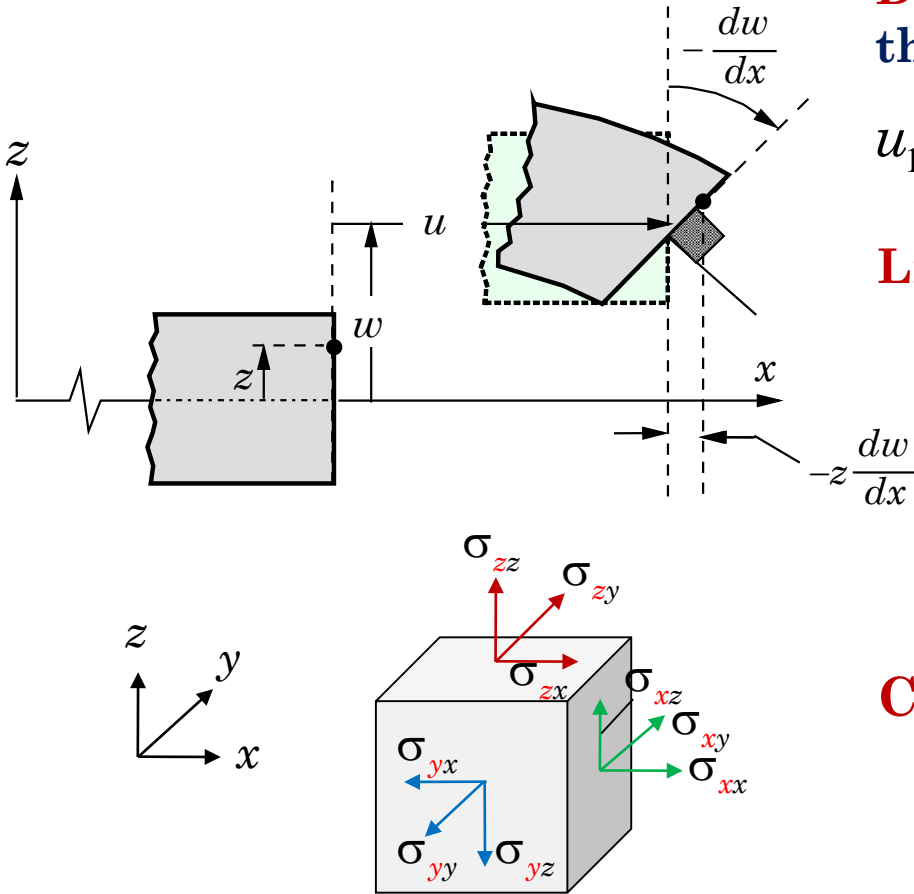
Strains, displacements, and rotations are small



Undeformed Beam

Euler-Bernoulli Beam Theory (EBT) is based on the assumptions of
(1) straightness,
(2) inextensibility, and
(3) normality

Kinematics of Deformation in the Euler-Bernoulli Beam Theory (EBT)



Displacement field (constructed using the hypothesis)

$$u_1(x, z) = u - z \frac{dw}{dx}, \quad u_2 = 0, \quad u_3 = w(x)$$

Linear strains

$$\varepsilon_{11} = \varepsilon_{xx} = \frac{\partial u_1}{\partial x_1} = \frac{du}{dx} - z \frac{d^2w}{dx^2},$$

$$\gamma_{xz} = \frac{\partial u_1}{\partial x_3} + \frac{\partial u_3}{\partial x_1} = -\frac{dw}{dx} + \frac{dw}{dx} = 0.$$

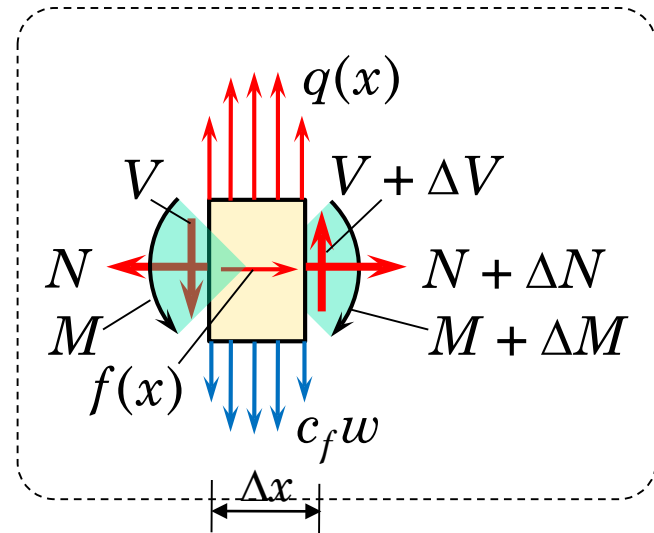
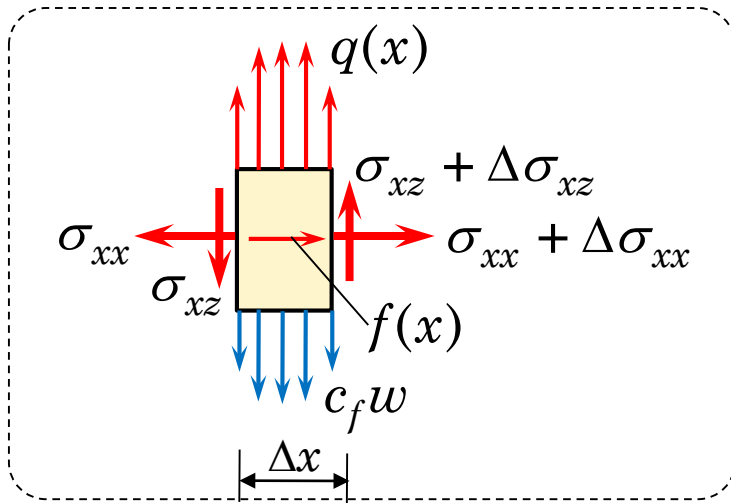
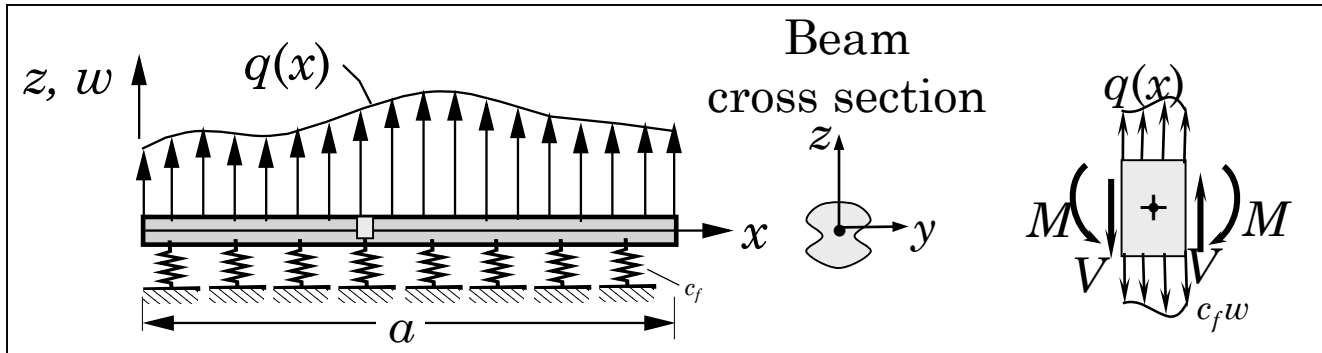
Constitutive relations

$$\sigma_{xx} = E \varepsilon_{xx} = E \frac{du}{dx} - Ez \frac{d^2w}{dx^2},$$

$$\sigma_{xz} = G \gamma_{xz} = 0$$

Notation for stress components

Euler-Bernoulli Beam Theory: Vector Approach



Definition of stress resultants

$$N = \int_A \sigma_{xx} dA, \quad M = \int_A \sigma_{xx} \cdot z dA, \quad V = \int_A \sigma_{xz} dA.$$

Euler-Bernoulli Beam Theory

Summation of forces in the x and z directions and moments about the y -axis.

$$\sum F_x = 0: -N + (N + \Delta N) + f\Delta x = 0$$

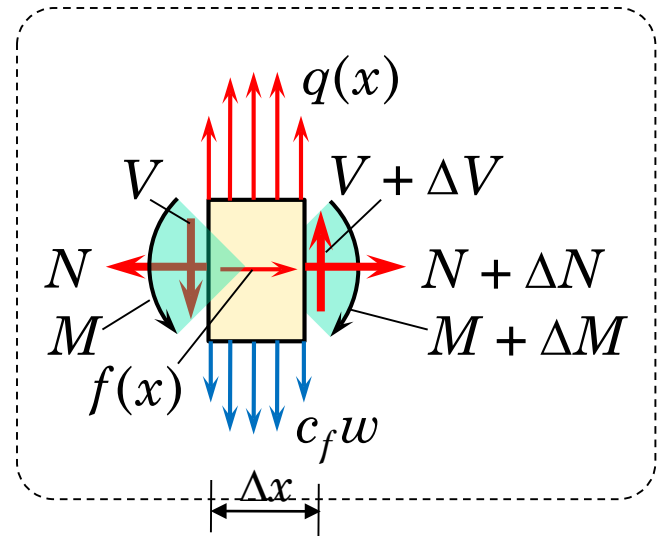
$$\lim_{\Delta x \rightarrow 0} \frac{\Delta N}{\Delta x} + f = 0 \Rightarrow \frac{dN}{dx} + f = 0$$

$$\sum F_z = 0: -V + (V + \Delta V) + q\Delta x - c_f w \Delta x = 0$$

$$\lim_{\Delta x \rightarrow 0} \frac{\Delta V}{\Delta x} + q - c_f w = 0 \Rightarrow \frac{dV}{dx} + q - c_f w = 0$$

$$\sum M_y = 0: -V\Delta x - M + (M + \Delta M) + (q\Delta x)\alpha\Delta x - (c_f w \Delta x)\beta\Delta x = 0$$

$$\lim_{\Delta x \rightarrow 0} -V + \frac{\Delta M}{\Delta x} + (q\Delta x)\alpha - (c_f w \Delta x)\beta = 0 \Rightarrow \frac{dM}{dx} - V = 0$$



Euler-Bernoulli Beam Theory (Continued)

Equilibrium equations

$$\frac{dN}{dx} + f = 0, \quad \frac{dM}{dx} - V = 0, \quad \frac{dV}{dx} + q - c_f w = 0$$

$$\int_A 1 \cdot dA = A, \quad \int_A z \cdot dA = 0, \quad \int_A z^2 \cdot dA = I$$

Stress resultants in terms of deflection

$$N = \int_A \sigma_{xx} dA = \int_A \left(E \frac{du}{dx} - Ez \frac{d^2w}{dx^2} \right) dA = EA \frac{du}{dx}$$

$$M = \int_A \sigma_{xx} \times z dA = \int_A \left(E \frac{du}{dx} - Ez \frac{d^2w}{dx^2} \right) z dA = -EI \frac{d^2w}{dx^2}$$

$$V = \frac{dM}{dx} = \frac{d}{dx} \left(-EI \frac{d^2w}{dx^2} \right)$$

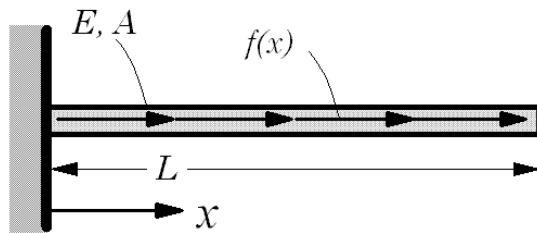
Euler-Bernoulli Beam Theory (Continued)

Governing equations in terms of the displacements

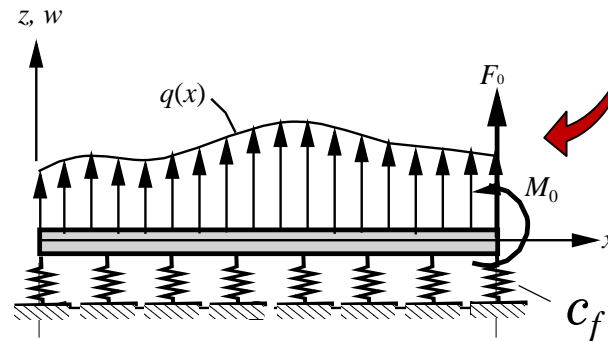
$$-\frac{d}{dx} \left(EA \frac{du}{dx} \right) - f = 0, \quad 0 < x < L$$

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 w}{dx^2} \right) + c_f w - q = 0, \quad 0 < x < L$$

Bars
 u



Axial deformation of a bar

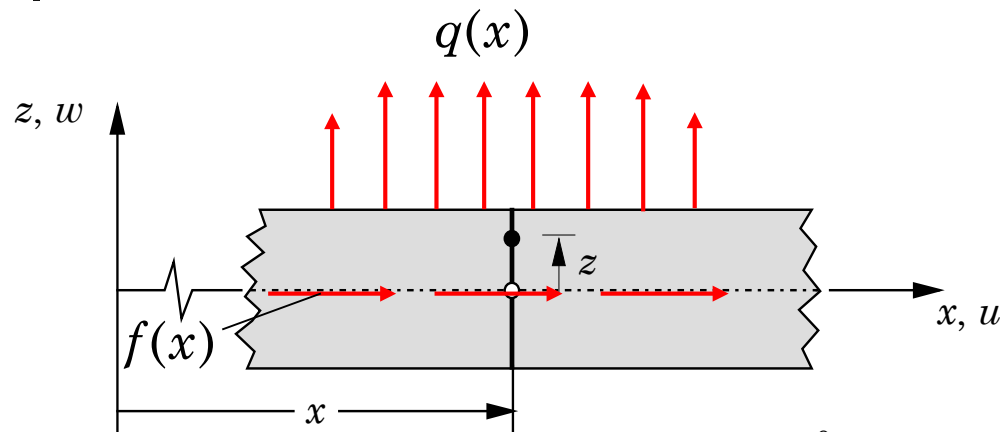


Bending of a beam

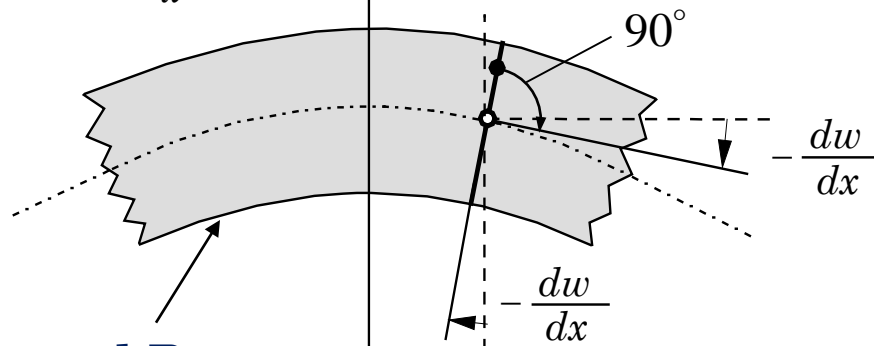
Beams
 w

Axial displacement is uncoupled from transverse displacement

Kinematics of Timoshenko Beam Theory

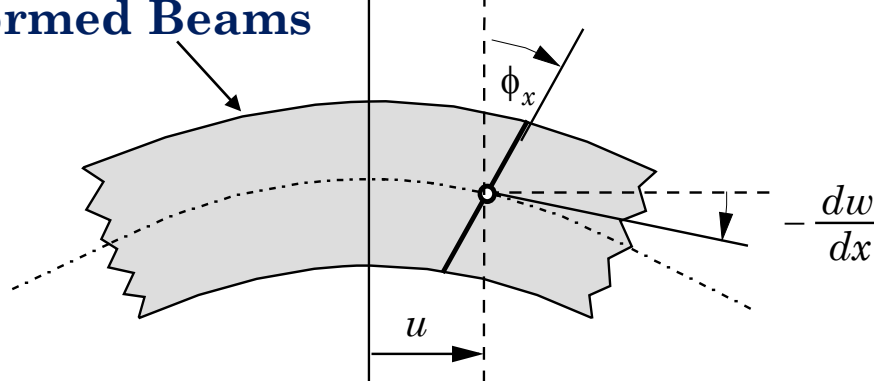


Undeformed Beam



Euler-Bernoulli Beam Theory (EBT)
Straightness, inextensibility, and normality

Deformed Beams



Timoshenko Beam Theory (TBT)
Straightness and inextensibility

Timoshenko Beam Theory

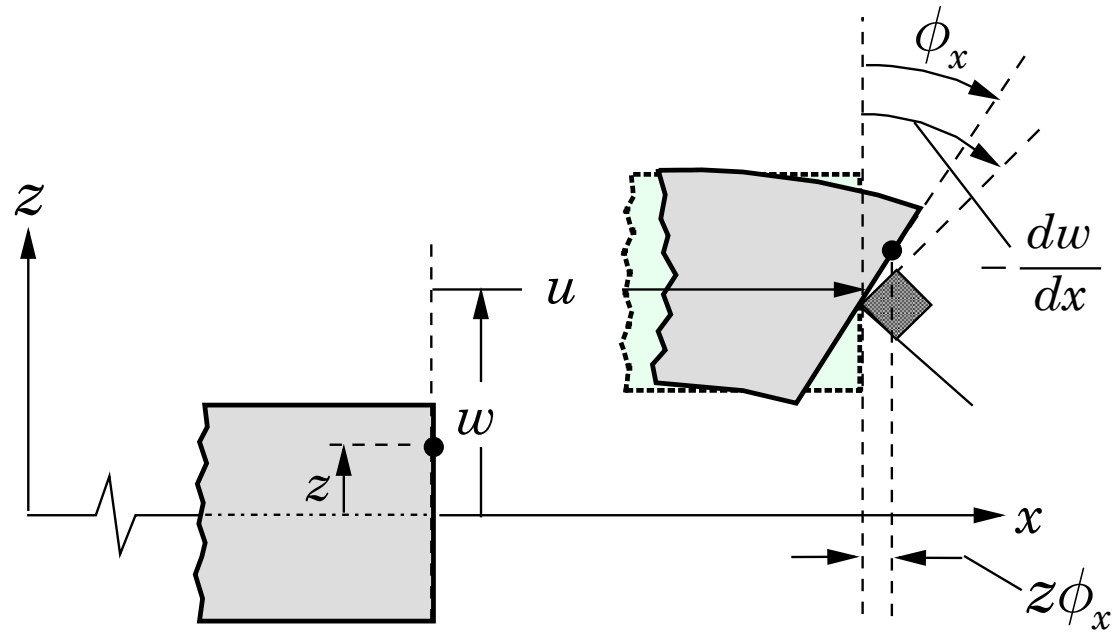
Kinematic Relations

$$u_1(x, z) = u(x) + z\phi_x(x),$$

$$u_2 = 0, \quad u_3(x, z) = w(x)$$

$$\varepsilon_{xx} = \frac{\partial u_1}{\partial x_1} = \frac{du}{dx} + z \frac{d\phi_x}{dx},$$

$$\gamma_{xz} = \frac{\partial u_1}{\partial x_3} + \frac{\partial u_3}{\partial x_1} = \phi_x + \frac{dw}{dx}$$



Constitutive Equations

$$\sigma_{xx} = E \varepsilon_{xx} = E \left(\frac{du}{dx} + z \frac{d\phi_x}{dx} \right)$$

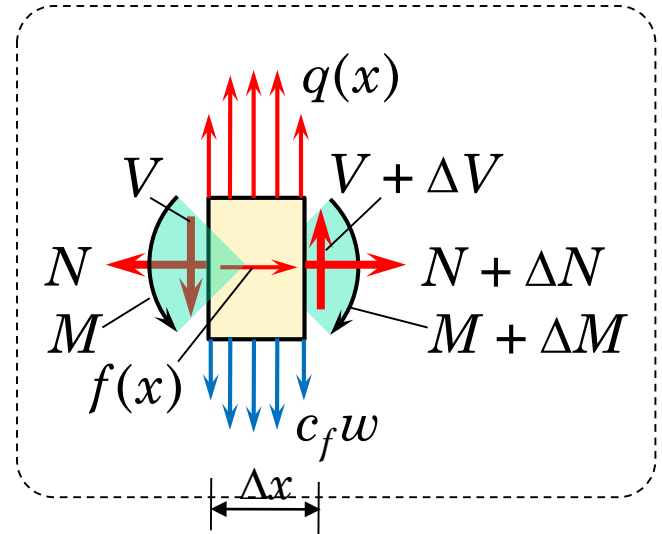
$$\sigma_{xz} = G \gamma_{xz} = G \left(\phi_x + \frac{dw}{dx} \right)$$

Timoshenko Beam Theory (Continued)

Equilibrium Equations (same as those from EBT)

$$\frac{dN}{dx} + f = 0, \quad -\frac{dV}{dx} - q + c_f w = 0,$$

$$-\frac{dM}{dx} + V = 0$$



Beam Constitutive Equations

$$N = \int_A \sigma_{xx} dA = \int_A E \left(\frac{du}{dx} + z \frac{d\phi_x}{dx} \right) dA = EA \frac{du}{dx}$$

$$M = \int_A \sigma_{xx} z dA = \int_A E \left(\frac{du}{dx} + z \frac{d\phi_x}{dx} \right) z dA = EI \frac{d\phi_x}{dx}$$

$$V = K_s \int_A \sigma_{xz} dA = GK_s \left(\phi_x + \frac{dw}{dx} \right) \int_A dA = GAK_s \left(\phi_x + \frac{dw}{dx} \right)$$

Timoshenko Beam Theory (Continued)

Governing Equations in terms of the displacements

$$-\frac{d}{dx} \left[GAK_s \left(\phi_x + \frac{dw}{dx} \right) \right] + c_f w = q \quad (1)$$

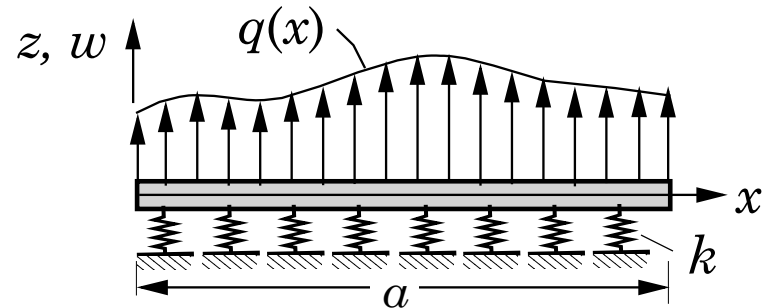
$$-\frac{d}{dx} \left(EI \frac{d\phi_x}{dx} \right) + GAK_s \left(\phi_x + \frac{dw}{dx} \right) = 0 \quad (2)$$

We have two second-order equations in two unknowns (w, ϕ_x). Next, we develop the weak forms over a typical beam finite element.

ANALYTICAL SOLUTIONS

Euler-Bernoulli beam theory (pure bending) – EI constant

$$EI \frac{d^4 w}{dx^4} + kw - q = 0, \quad 0 < x < a$$



Boundary conditions:

$$(w, V) \text{ and } (\theta_x, M); \quad \theta_x = -\frac{dw}{dx}$$

General solution for uniform load:

$$w(x) = \left(c_1 \sinh \frac{2\beta x}{a} + c_2 \cosh \frac{2\beta x}{a} \right) \sin \frac{2\beta x}{a} + \left(c_3 \sinh \frac{2\beta x}{a} + c_4 \cosh \frac{2\beta x}{a} \right) \cos \frac{2\beta x}{a} + \frac{q_0}{k} \quad \beta = \frac{k a^4}{64 EI}$$

c_1 thru c_4 are constants to be determined using boundary conditions.

ANALYTICAL SOLUTIONS (continued)

Simply supported beam: Using symmetry and half beam,

$$\frac{dw}{dx} = \frac{d^3w}{dx^3} = 0 \text{ at } x = 0; \quad w = \frac{d^2w}{dx^2} = 0 \text{ at } x = \frac{a}{2}$$

We obtain $c_2 = c_3 = 0$, and

$$c_1 \sin \beta \sinh \beta + c_4 \cos \beta \cosh \beta + \frac{q_0}{k} = 0,$$

$$c_1 \cos \beta \cosh \beta + c_4 \sin \beta \sinh \beta = 0.$$

Solving these equations, we obtain

$$c_1 = -\frac{2q_0}{k} \left(\frac{\sin \beta \sinh \beta}{\cos 2\beta + \cosh 2\beta} \right), \quad c_4 = -\frac{2q_0}{k} \left(\frac{\cos \beta \cosh \beta}{\cos 2\beta + \cosh 2\beta} \right)$$

Similarly, one can obtain solutions for other boundary conditions.

ANALYTICAL SOLUTIONS (by integration)

Solution with $k = 0$: Successive integrations yield

$$\frac{d}{dx} \left(EI \frac{d^2 w}{dx^2} \right) = \int q(x) dx + C_1,$$

$$EI \frac{d^2 w}{dx^2} = \int \left(\int q(x) dx \right) dx + C_1 x + C_2,$$

$$\frac{dw}{dx} = \int \frac{1}{EI} \left[\int \left(\int q(x) dx \right) dx \right] dx + C_1 \int \frac{x}{EI} dx + C_2 \int \frac{1}{EI} dx + C_3,$$

$$w = \int \left\{ \int \frac{1}{EI} \left[\int \left(\int q(x) dx \right) dx \right] dx \right\} dx + C_1 \int \left(\int \frac{x}{EI} dx \right) dx \\ + C_2 \int \left(\int \frac{1}{EI} dx \right) dx + C_3 x + C_4.$$

The constants of integration can be determined using the boundary conditions

ANALYTICAL SOLUTIONS (by integration)

Simply supported beam under uniform load:

The boundary conditions on the moment give

$$M(0) = M(a) = 0 \Rightarrow C_2 = 0, C_1 = -\frac{q_0 a}{2},$$

$$w(x) = \frac{q_0 x^4}{24EI} - \frac{q_0 a x^3}{12EI} + C_3 x + C_4.$$

$$w(0) = 0 \quad w(a) = 0 \Rightarrow C_4 = 0, C_3 = (q_0 a^3 / 24EI).$$

The solutions for the bending moment and deflection become

$$M(x) = -EI \frac{d^2 w}{dx^2} = \frac{q_0}{2} x(a - x), \quad 0 < x < a.$$

$$w(x) = \frac{q_0 x^4}{24EI} - \frac{q_0 a x^3}{12EI} + \frac{q_0 x a^3}{24EI} = \frac{q_0}{24EI} (x^4 - 2x^3 a + a^3 x).$$

The maximum values are

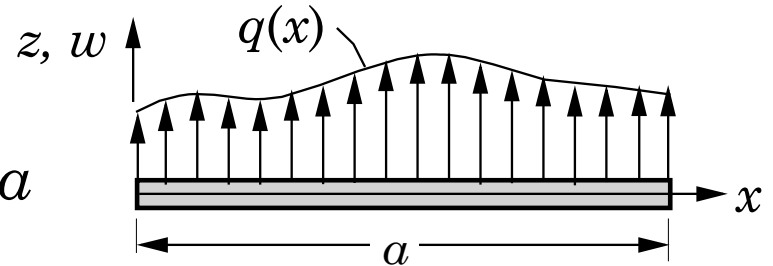
$$w_{\max} = w(0.5a) = \frac{5q_0 a^4}{384EI}; \quad M(0.5a) = \frac{q_0 a^2}{8}$$

NAVIER SOLUTIONS

for simply supported beams

Euler-Bernoulli beam theory (pure bending) – EI constant

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 w}{dx^2} \right) + kw - q = 0, \quad 0 < x < a$$



Simply supported: $w = 0, \quad M \equiv -EI \frac{d^2 w}{dx^2} = 0$ at $x = 0, a$

Solution form: $w = \sum_{n=1}^{\infty} W_n \sin \alpha_n x, \quad \alpha_n = \frac{n\pi}{a}$

$$\frac{d^2 w}{dx^2} = - \sum_{n=1}^{\infty} \alpha_n^2 W_n \sin \alpha_n x$$



Navier's Solution of Simply supported Beams

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 w}{dx^2} \right) + kw - q = 0$$

$$\Rightarrow \sum_{n=1}^{\infty} W_n \left[(-1)^2 EI \alpha_n^4 + k \right] \sin \alpha_n x = q(x)$$

$$q(x) = \sum_{n=1}^{\infty} Q_n \sin \alpha_n x, \quad Q_n = \frac{2}{a} \int_0^a q(x) \sin \alpha_n x \, dx$$

$$\sum_{n=1}^{\infty} \left\{ W_n \left[(-1)^2 EI \alpha_n^4 + k \right] - Q_n \right\} \sin \alpha_n x = 0$$

$$W_n = \frac{Q_n}{\left[(-1)^2 EI \alpha_n^4 + k \right]}$$

$$w = \sum_{n=1}^{\infty} W_n \sin \alpha_n x, \quad \alpha_n = \frac{n\pi}{a}$$